

PERSPECTIVE OF FOOT BRIDGE



PERSPECTIVE OF BOARDWALK WITH REST AREA & STILT TYPE GAZEBO -1

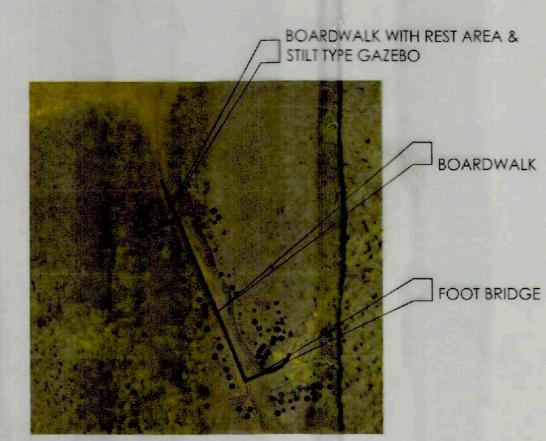


PERSPECTIVE OF BOARDWALK WITH REST AREA & STILT TYPE GAZEBO-2



THIS SITE ARTECHE WESTERN SAMAR EASTERN SAMAR





VICINITY PLAN GC-01 SCALE

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TOURISM INFRASTRUCTURE AND ENTERPRISE ZONE AUTHORITY 6th & 7th FLR. TOWER 1 DOUBLE DRAGON PLAZA
DOUBLE DRAGON MERIDIAN PARK, MACAPAGAL AVENUE
CORNER EDSA EXTENSION BAY AREA PASAY CITY 1308

CONSTRUCTION OF THE MANGROVE FOREST TRAIL AND CONSERVATION CENTER ARTECHE, EASTERN SAMAR

PROJECT TITLE/LOCATION

PREPARED BY: TRUBBY D. EVASCO DRAWN BY: TDE DATE: JANUARY 2024

PROJECT EVALUATION AND PLANNING DEPARTMENT REVIEWED BY: CHECKED BY: RADY EL ADAME JEOFFREY L. MACALALAD MANAGER PROJECT PLANNING & DESIGN DIVISION DEPARTMENT MANAGER
PROJECT EVALUATION & PLANNING DEPARTMENT

RECOMMENDING APPROVAL: ASSISTANT CHIEF OPERATING OFFICER ARCHITECTURAL & ENGINEERING SERVICES SECTOR

CHIEF OPERATING OFFICER

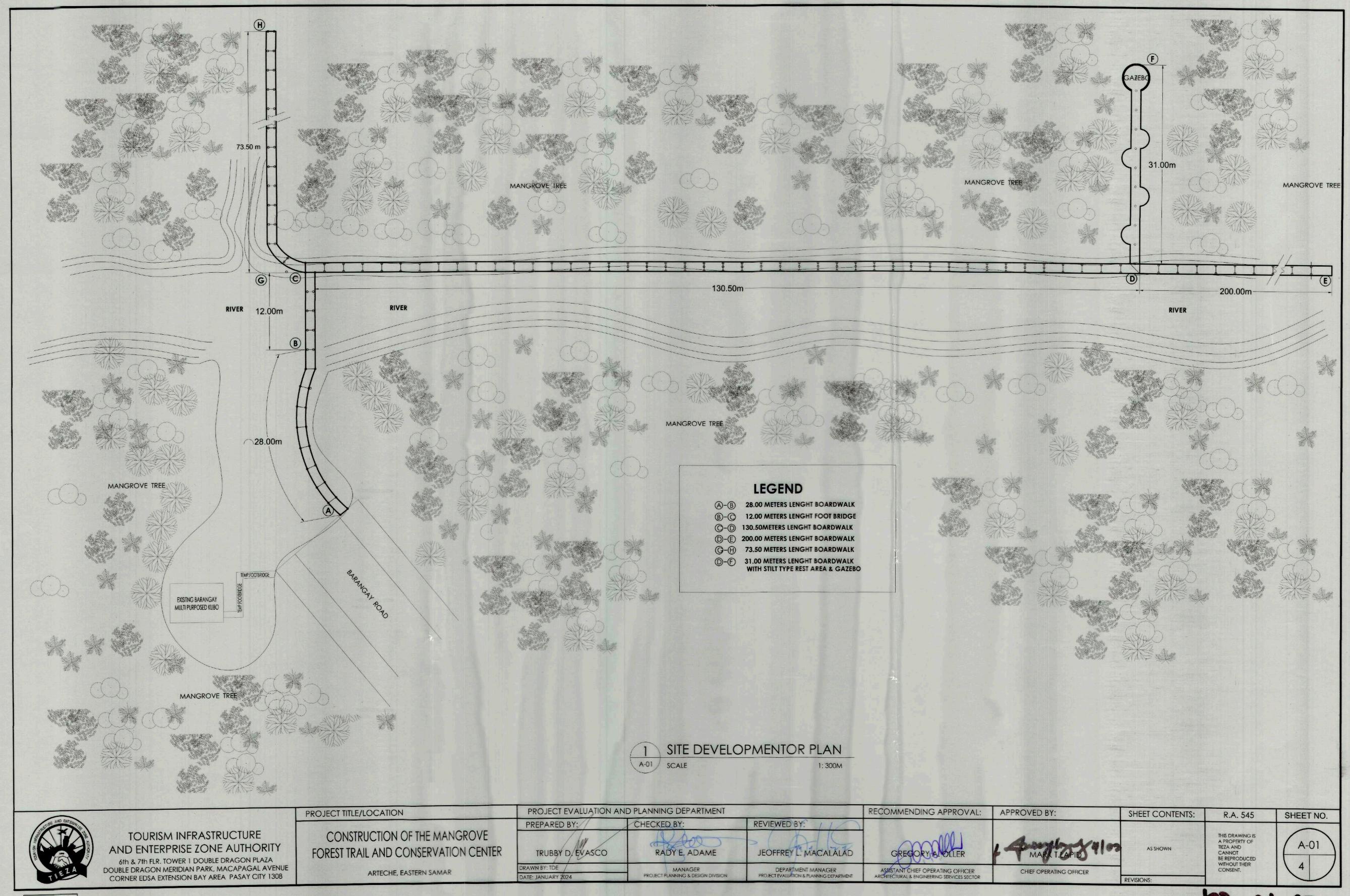
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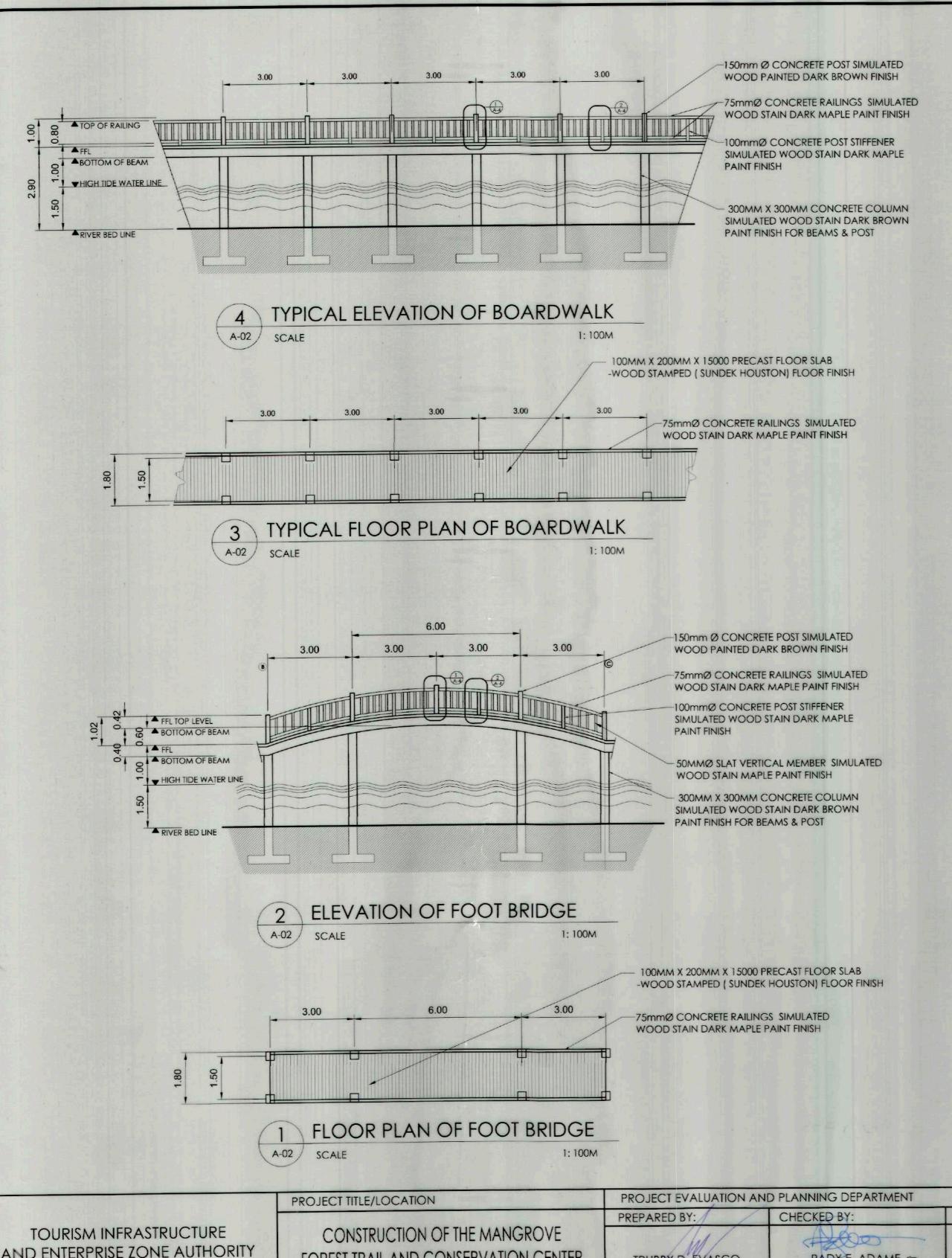
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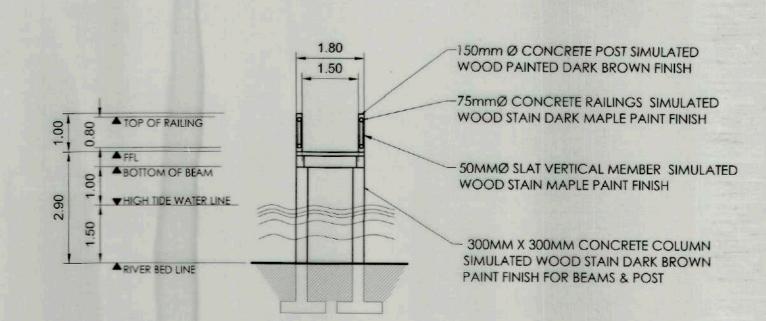
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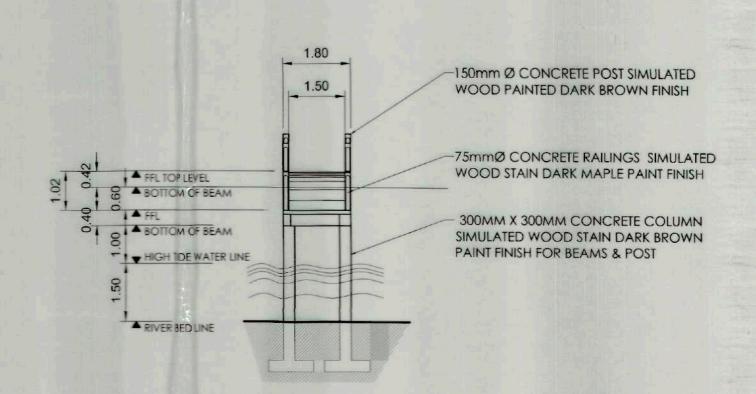
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TYPICAL SECTION OF BOARDWALK



SECTION OF FOOT BRIDGE A-02



AND ENTERPRISE ZONE AUTHORITY

6th & 7th FLR. TOWER 1 DOUBLE DRAGON PLAZA DOUBLE DRAGON MERIDIAN PARK, MACAPAGAL AVENUE CORNER EDSA EXTENSION BAY AREA PASAY CITY 1308 FOREST TRAIL AND CONSERVATION CENTER ARTECHE, EASTERN SAMAR

REVIEWED BY: RADY E. ADAME JEOFFREY L. MACALALAD TRUBBY D. EVASCO DRAWN BY: TDE MANAGER PROJECT PLANNING & DESIGN DIVISIO DEPARTMENT MANAGER
PROJECT EVALUATION & PLANING DEPAR DATE: JANUARY 2024

RECOMMENDING APPROVAL ASSISTANT CHIEF OPERATING OFFICER ARCHITECTURAL & ENGINEERING SERVICES SECTOR

APPROVED BY: CHIEF OPERATING OFFICER

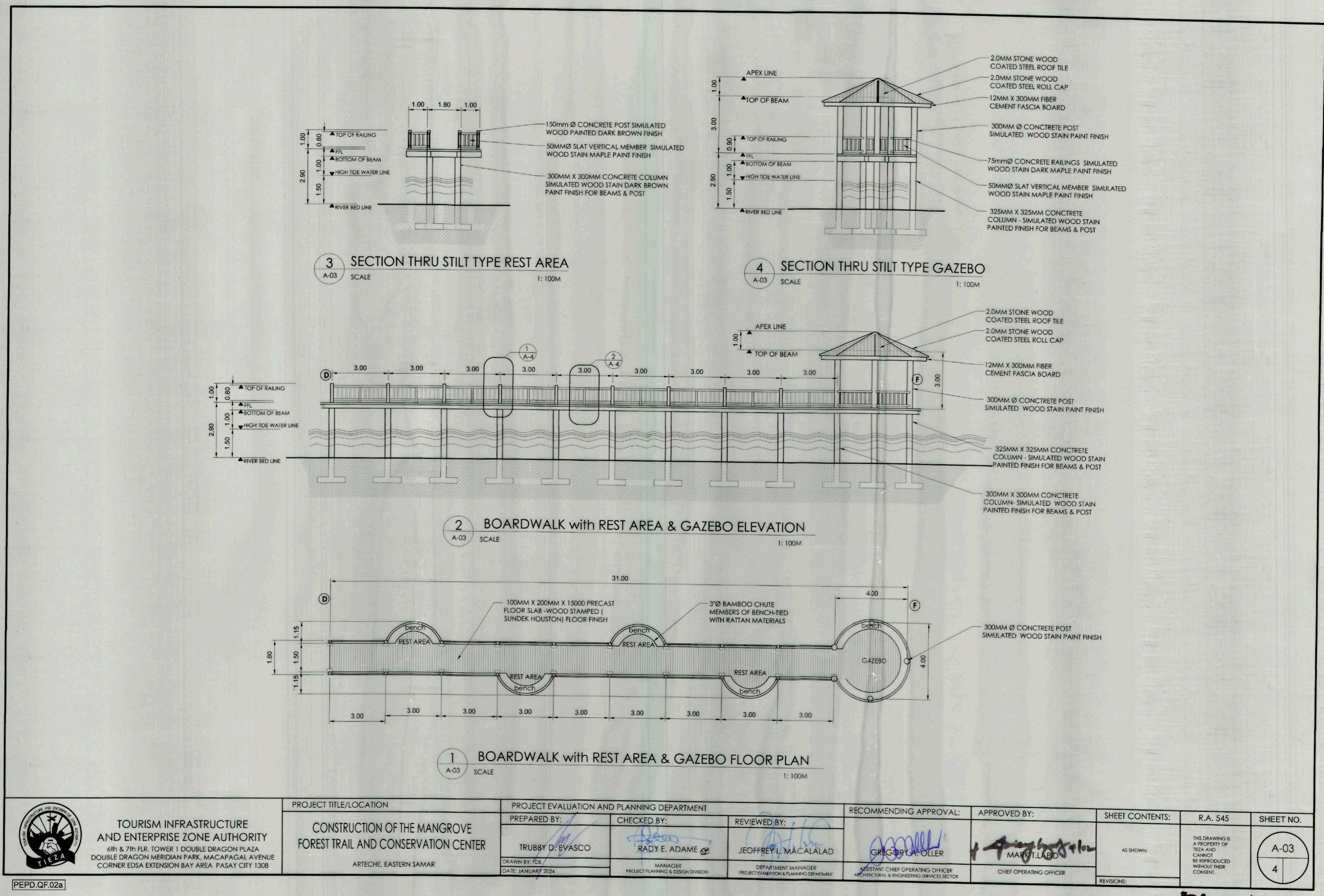
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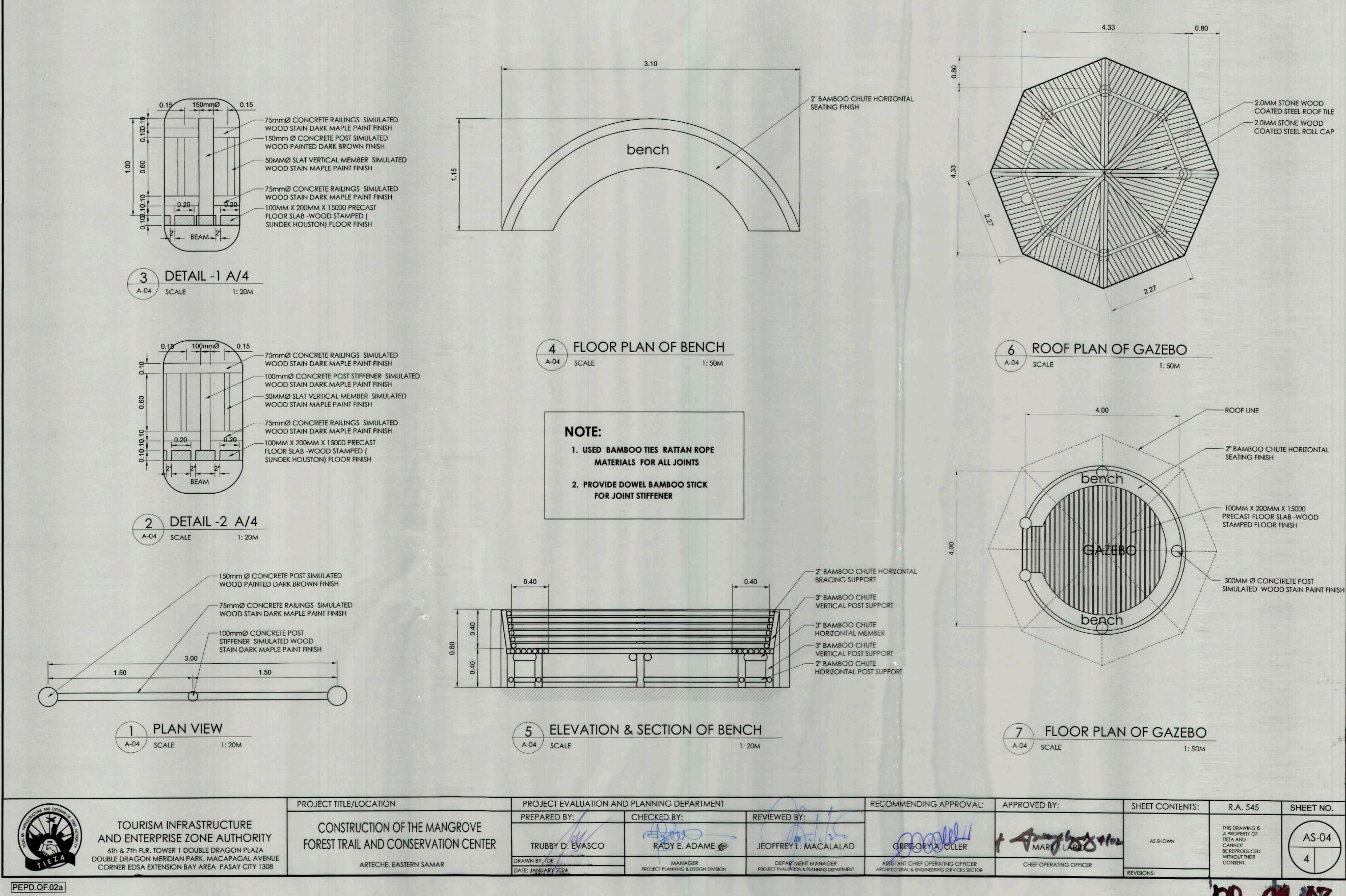
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SHEET NO.





STRUCTURAL NOTES

A. GENERAL

A.1 IN THE INTERPRETATION OF THESE DRAWINGS, INDICATED DIMENSION SHALL GOVERN. DISTANCES OR SIZES SHALL NOT BE SCALED FOR CONSTRUCTION PURPOSES.

A.2 IN CASE OF CONFLICT IN DETAILS OR DIMENSIONS BETWEEN THE ARCHITECTURAL AND STRUCTURAL PLANS, THE CONTRACTOR SHALL RELEASE A REQUEST FOR INFORMATION (RFI) THAT SHALL BE RESPONDED BY BOTH THE ARCHITECT-OF-RECORD AND THE SIGNING STRUCTURAL ENGINEER.

A.3 IN CASES OF CONFLICT BETWEEN THE STRUCTURAL PLANS AND THE GENERAL NOTES AND SPECIFICATIONS, THE CONTRACTOR SHALL RELEASE A REQUEST FOR INFORMATION (RFI) THAT SHALL BE RESPONDED BY THE SIGNING STRUCTURAL ENGINEER.

A.4 ALL STRUCTURAL DETAILS UNLESS OTHERWISE SHOWN IN THE DRAWINGS SHALL FOLLOW CONSTRUCTION NOTE DETAILS.

A.5 THE CONTRACTOR MUST ENSURE AND WILL BE HELD RESPONSIBLE FOR THE OVERALL STABILITY OF THE BUILDING/STRUCTURE/EXCAVATION AT ALL STAGES OF THE WORK.

A.6 ALL STRUCTURAL WORKS SHALL BE CARRIED OUT IN COMPLIANCE WITH THE REQUIREMENTS OF THE NATIONAL STRUCTURAL CODE OF THE PHILIPPINES 2015.

A.7 THE CONTRACTOR SHALL COORDINATE WITH THE ARCHITECT, MECHANICAL, ELECTRICAL, STRUCTURAL ENGINEER PLANS AS TO THE EXACT SIZE AND LOCATION OF OPENINGS THRU FLOOR SLABS, BEAMS AND WALLS.

B. CONCRETE

B.1 UNLESS OTHERWISE INDICATED IN PLANS OR NOTED IN THE SPECIFICATIONS, THE MINIMUM 28-DAYS CYLINDER COMPRESSIVE STRENGTH OF CONCRETE, fc', SHALL BE AS FOLLOWS

B.1.1 FOUNDATION21 MPa (3000 psi)

- B.1.2 COLUMNS21 MPa (3000 psi)
- B.1.3 SUSPENDED SLABS, BEAMS & GIRDERS21 MPa (3000 psi)
- B.1.4 PARAPET WALLS & OTHER STRUCTURALELEMENTS21 MPa (3000 psi)
- B.1.5 PARTITIONS, CURTAIN WALLS, BEDDEDSLABS, SIDEWALKS, CURB, GUTTERAND OTHER

STRUCTURAL ELEMENTS 17 MPa (2500 psi)

B.1.6LEAN CONCRETE 10 MPa (1500 psi)

B.2 NO DEPOSITING OF CONCTETE SHALL BE ALLOWED WITHOUT THE USE OF VIBRATORS UNLESS AUTHORIZED IN WRITING BY THE DESIGNERS AND ONLY FOR UNUSUAL CONDITIONS WHERE VIBRATIONS IS EXTREMELY DIFFICULT TO ACCOMPLISH.

C. REINFORCING STEEL BARS

- C.1 ALL REINFORCING BARS SHALL BE DEFORMED STEEL BARS IN ACCORDANCE WITH ASTM A-615.
- C.2 ALL REINFORCING DEFORMED STEEL BARS SHALL HAVE A MINIMUM YIELD STRENGTH OF 276

MPa (40 ksi).

- C.3 CONCRETE COVER
- C.3.1 CAST AGIANTS AND PERMANENTLY IN CONTACT WITH GROUND (75 MM)
- C.3.2 EXPOSED TO WEATHER OR IN CONTACT WITH GROUND
 - C.3.2.1. 20MMØ THROUGH 58MMØ BARS (50 MM)
 - C.3.2.2. 16MMØ AND SMALLER BARS (40 MM)
- C.3.3. NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND
 - C.C.3.1.SLABS, JOISTS, AND WALLS

40MMØ AND 58MMØ BARS (40 MM)

36MMØ BARS AND SMALLER (20 MM)

C.3.3.2. BEAMS, COLUMNS, PEDESTALS AND TENSION TIES (40 MM)

C.4 REINFORCING BAR SPLICES

- C.4.1 FOR BEAMS ALL SPLICES SHALL BE LAPPED OT EXTENDED INTO MINIMUM OF 50D UNLESS SHOWN OTHERWISE. HORIZONTAL SPLICES SHALL BE STAGGERED FOR BEAMS AND GIRDERS.
- C.4.2 FOR COLUMNS: ALL SPLICES SHALL BE WELDED OR LAPPED LOCATED NEAR THE MIDHEIGHT LEVEL. NO MORE THAN 50% OF THE TOTAL NUMBER OF MAIN BARS SHALL BE SPLICED AT ANY SAME LEVEL LOCATION.

C.5BUNDLED REINFORCEMENTS

- C.5.1BUNDLED BARS SHALL BE ENCLOSED WITHIN TRANSVERSE REINFORCEMENT. BUNDLED BARS IN COMPRESSION MEMBERS SHALL BE ENCLOSED BY TRANSVERSE REINFORCEMENT AT LEAST 12MMØ IN SIZE.
- C.5.2BARS LARGER THAN A 36MMØ SHALL NOT BE BUNDLED IN BEAMS.
- C.5.3DEVELOPMENT LENGTH FOR INDIVIDUAL BARS WITHIN A BUNDLE, IN TENSION OR COMPRESSION, SHALL BE THAT OF INDIVIDUAL BAR, INCREASED 20 PERCENT FOR A THREE-BAR BUNDLE, AND 33 PERCENT FOR A FOUR-BAR BUNDLE.
 - D. STRUCTURAL STEELD.1ALL STRUCTURAL STEEL, BOTH ANGLES AND STEEL PLATES SHALL CONFORM TO ASTM A36 AND SHALL HAVE A MINUMUM YIELD STRENGTH OF 248 MPa (36 ksi).D.2WELDING ELECTRODES SHALL BE E70XX SERIES.D.3MACHINE AND ACNHOR BOLTS SHALL CONFORM TO ASTM A-325, UNLESS NOTED OTHERWISE ON DRAWINGS.D.4STEEL ANCHOR RODS SHALL CONFORM TO ASTM A-36, UNLESS NOTED OTHERWISE ON DRAWINGS.

D. FOUNDATION

D.1 THE FOUNDATION SYSTEM OF THE STRUCTURE IS BASED ON THE ASSUMED SOIL BEARING CAPACITY (SBC) OF 100 KPA. NO SOIL INVESTIGATION ARE CONDUCTED PRIOR TO THE FOUNDATION DESIGN. THE CONTRACTOR SHALL CONFIRM THE APPLICABILITY OF THE SPECIFIED FOUNDATION TYPE BASED ON THE ACTUAL SITE CONDITION. THE FOUNDATION SHALL BE REVISED IF NECESSARY AS DICTATED BY ACTUAL SITE CONDITION.

D.2 EXISTING UNDERGROUND PIPES. TUNNELS, ETC., SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR EVALUATION.

D.3 AN EXCAVATION ADJACENT TO AN EXISTING STRUCTURE SHALL BE PROVIDED WITH ADEQUATE SHEET PILING BY THE CONTRACTOR. THE SHEET PILES SHALL BE PROPERLY DESIGNED TO RESIST EARTH AND WATER PRESSURE AS WELL AS SURCHARGE LOADING ON THE FOOTING OF THE ADJACENT EXISTING STRUCTURE.

NOTE: FOR fc'=21MPa (3000psi), fy=276MPa (40ksi)

	MINIMUM DEVELOPMENT AND SPLICE LENGTHS (mm)										
	TENSION										
BAR						TOP BARS					
DIAMETER (mm)	Ld	HOOK Ldh	CLASS A SPLICE=Ld	CLASS B SPLICE=1.3Ld	Ld	CLASS A SPLICE=Ld	CLASS B SPLICE=1.3Ld	Ldc	SPLICE		
10	300	150	300	390	390	390	510	200	300		
12	345	175	345	450	450	450	585	200	300		
16	460	235	460	600	600	600	780	235	315		
20	575	290	575	750	750	750	975	290	395		
25	890	365	890	1160	1160	1160	1510	365	490		
2 28	995	405	995	1295	1295	1295	1685	405	550		
32	. 1135	465	1135	1480	1480	1480	1925	465	630		
36	1280	525	1280	1665	1665	1665	2165	525	710		

TABLE 3: TABLE OF LAP SPLICES AND DEVELOPMENT LENGTHS

TYPE OF STANDARD HOOK	BAR SIZE	MIN. INSIDE BEND DIA. (mm)	STRAIGHT EXTENSION ^[1] Lext (mm)	TYPE OF STANDARD HOOK
90-DEGREE	10mmØ THROUGH 16mmØ	4db	GREATER OF 6db AND 75mm	90-DEGREI BEND
HOOK	20mmØ THROUGH 25mmØ	6db	12db	DIAMETER Lex
135-DEGREE	10mmØ THROUGH 16mmØ	4db	GREATER OF	db 135-DEGR BEND
HOOK	20mmØ THROUGH 25mmØ	6db	6db AND 75mm	DIAMETER -
180-DEGREE	10mmØ THROUGH 16mmØ	4db	GREATER OF	db 180-DEGRE
HOOK	20mmØ THROUGH 25mmØ	6db	4db AND 65mm	DIAMETER

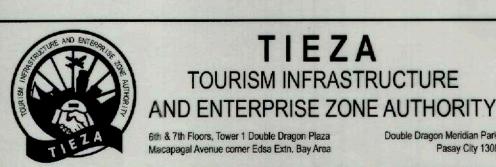
[1] A STANDARD HOOK FOR STIRRUPS, TIES, AND HOOPS INCLUDES THE SPECIFIC INSIDE BEND DIAMETER AND STRAIGHT EXTENSION LENGTH. IT SHALL BE PERMITTED TO USE A LONGER STRAIGHT EXTENSION AT THE END OF A HOOK. A LONGER EXTENSION SHALL NOT BE CONSIDERED TO INCREASE THE ANCHORAGE CAPACITY OF THE HOOK.

TABLE 2: MINIMUM INSIDE BEND DIAMETERS AND STANDARD HOOK GEOMETRY FOR STIRRUPS, TIES, AND HOOPS

TYPE OF STANDARD HOOK	BAR SIZE	MIN. INSIDE BEND DIA. (mm)	STRAIGHT EXTENSION ^[1] Lext (mm)	TYPE OF STANDARD HOOK		
	10mmØ THROUGH 25mmØ	6db		POINT AT WHICH BAR IS DEVELOPED 90-DEGR		
90-DEGREE HOOK	28mmØ THROUGH 36mmØ	8db	12db	DIAMETER Lext		
	40mmØ THROUGH 58mmØ	10db		Ldh		
	10mmØ THROUGH 25mmØ	6db		POINT AT WHICH BAR IS DEVELOPED 180-DEGR BEND		
180-DEGREE HOOK	28mmØ THROUGH 36mmØ	8db	GREATER OF 4db AND 65mm	DIAMETER		
	40mmØ THROUGH 58mmØ	10db		Ldh Lext		

[1] A STANDARD HOOK FOR DEFORMED BARS IN TENSION INCLUDES THE SPECIFIC INSIDE BEND DIAMETER AND STRAIGHT EXTENSION LENGTH. IT SHALL BE PERMITTED TO USE A LONGER STRAIGHT EXTENSION AT THE END OF A HOOK. A LONGER EXTENSION SHALL NOT BE CONSIDERED TO INCREASE THE ANCHORAGE CAPACITY OF THE HOOK.

TABLE 3: STANDARD HOOK GEOMETRY FOR DEVELOPMENT OF DEFORMED BARS IN TENSION



CONSTRUCTION OF THE MANGROVE
FOREST TRAIL AND CONSERVATION CENTER
ARTECHE, EASTERN SAMAR

PROJECT EVALUATION AND PLANNING DEPARTMENT
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DRAWN BY: MEPR

DATE:

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SHEET CONTENTS:

STRUCTURAL NOTES

REVISIONS:

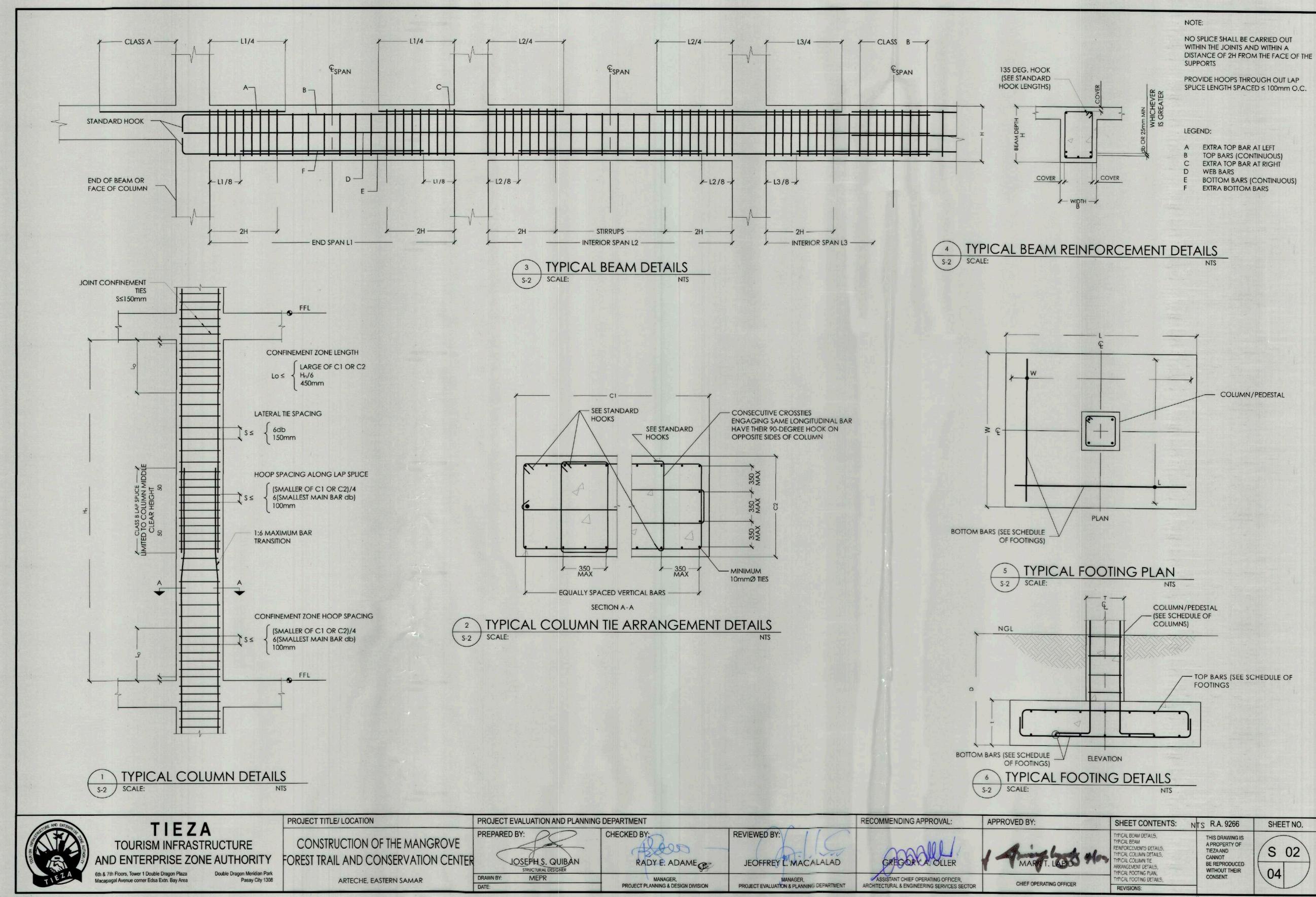
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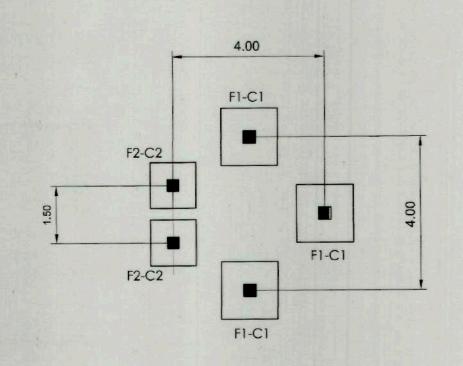
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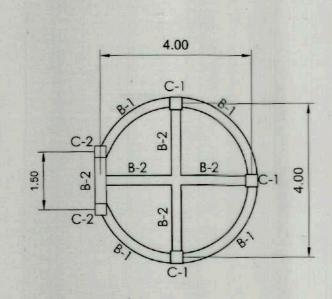
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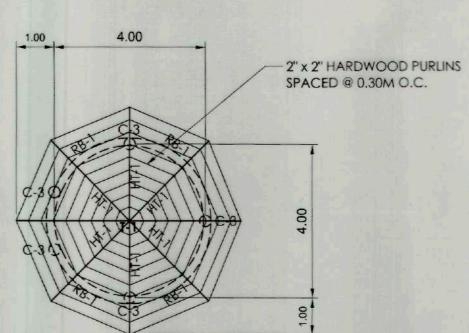
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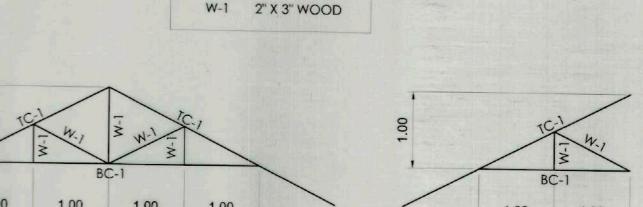




REVIEWED BY:

JEOFFREY L. MACALALAD

DEPARTMENT MANAGER
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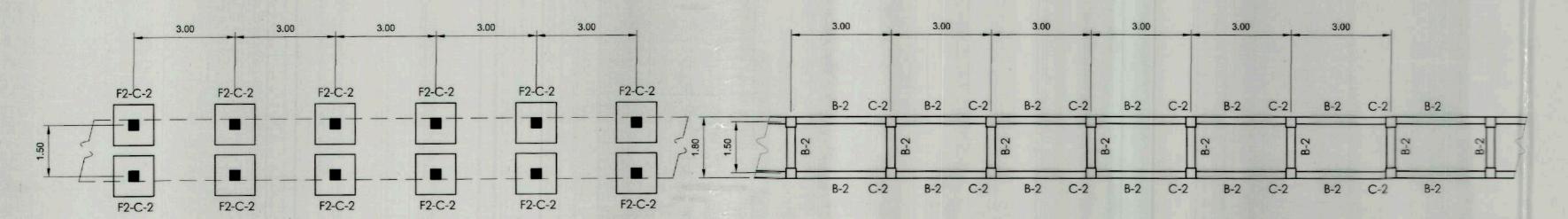
TC-1 2" X 3" WOOD BC-1 2" X 3" WOOD

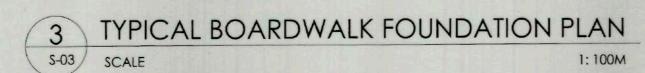
6 STILT GAZEBO FOUNDATION PLAN
1: 100M

7 STILT GAZEBO FLOOR FRAMING PLAN
1: 100M

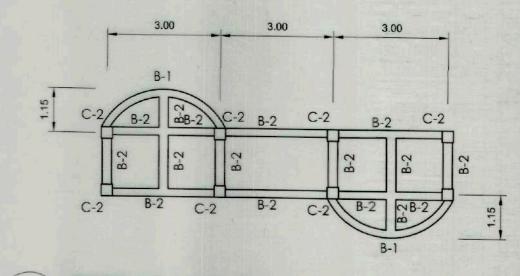
8 STILT GAZEBO ROOF FRAMING PLAN
1: 100M

9 STILT GAZEBO TRUSS DETAIL
S-03 SCALE 1: 50M

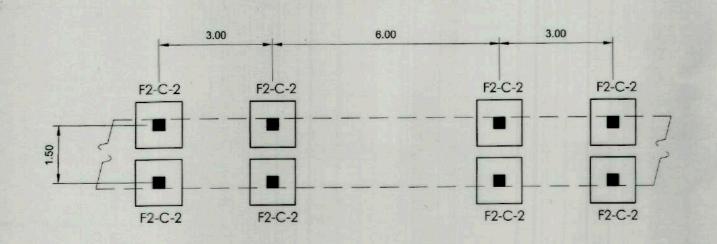


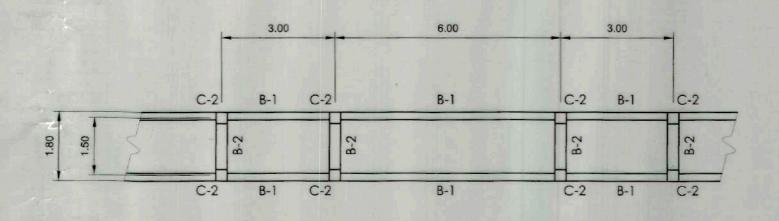


4 TYPICAL BOARDWALK FLOOR FRAMING PLAN
S-03 SCALE 1: 100M



5 REST AREA FLOOR FRAMING PLAN
1: 100M





FOOT BRIDGE FOUNDATION PLAN
S-03 SCALE 1:100M

2 FOOT BRIDGE FLOOR FRAMING PLAN
1: 100M



TOURISM INFRASTRUCTURE

AND ENTERPRISE ZONE AUTHORITY

6th & 7th FLR. TOWER 1 DOUBLE DRAGON PLAZA

DOUBLE DRAGON MERIDIAN PARK, MACAPAGAL AVENUE

CORNER EDSA EXTENSION BAY AREA PASAY CITY 1308

CONSTRUCTION OF THE MANGROVE
FOREST TRAIL AND CONSERVATION CENTER
ARTECHE, EASTERN SAMAR

PROJECT EVALUATION AND PLANNING DEPARTM				
PREPARED BY:	CHECKED BY:			
JOSEPH S. QUIBAN STRUCTURAL DESIGNER	RADY E. ADAME			
PRAWN BY: MEPR	MANAGER PROJECT PLANNING & DESIGN DIVISION			
41.00				



MARK T. LAPID

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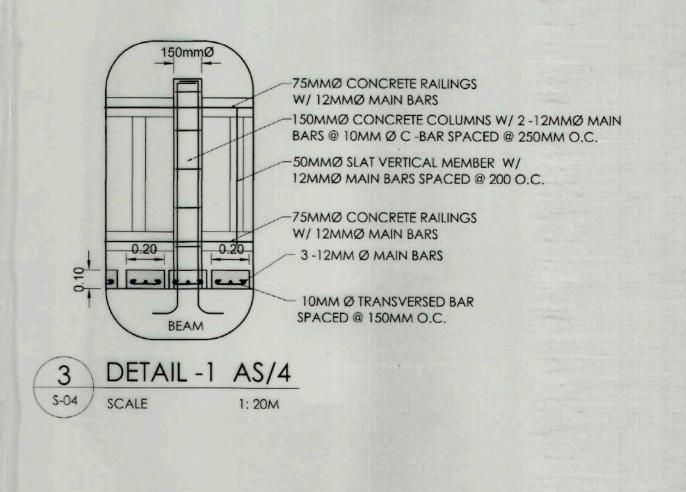
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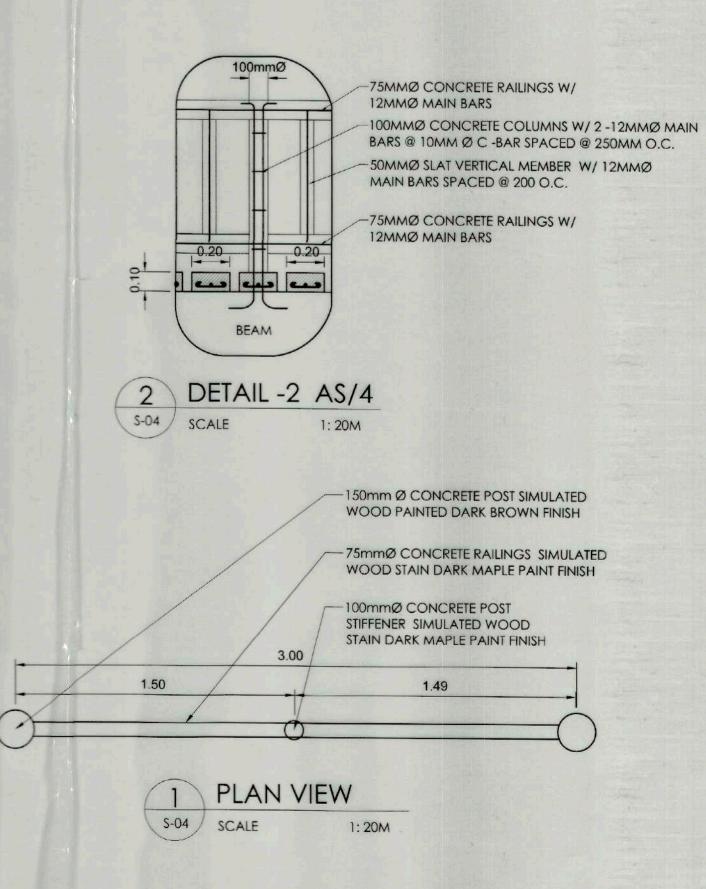
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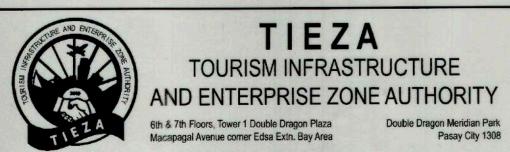
				SCHEDUL	E OF CC	LUMNS			
		DIMENSIO	201 (1414)		REI	NFORCEMEN	ITS		
LEVEL	MARK	DIMENSION (MM)			MAIN BAR	LATERAL TIES			
		В	Н	SECTION	IVIAIIV BAK	DIAMETER	SET OF TIES	SPACING	
BOARDWALK TO FOUNDATION	C-1	0.325	0.325	H B	8-16MMØ	12MMØ	3	1@50, 7@100, REST@150	
BOARDWALK TO FOUNDATION	C-2	0.30	0.30	H B	8-16MMØ	12MMØ	3	1@50, 7@100, REST@150	
ROOF TO BOARDWALK	C-3	0.3 (DIAM		B	6-16MMØ	12MMØ	1	1@50, 4@100, REST@150	

		S	CHEDULE	OF FO	DOTINGS	3			
		DIMENS	ION (MM)		REINFORCEMENTS				
MARK	LENGTH	WIDTH	THICKNESS	DEPTH	BOTTOM BARS		TOP BARS		
	L	W	T	D	ALONG L	ALONG W	ALONG L	ALONG W	
F-1	1.50	1.50	0.40	1.50	16Ø @ 0.20	16Ø @ 0.20			
F-2	1.20	1.20	0.40	1.50	16Ø @ 0.20	16Ø @ 0.20			

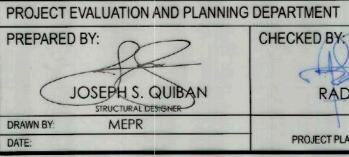
					SCHED	ULE OF BE	AMS			
15./5	MADK	DIMENSI	(MM) NO		R	EINFORCEMENTS				STIRRUPS
LEVEL	MARK	В	н	BAR POSITION	SUP	PORT	MID	SPAN	SIZE	SPACING
				TOP BAR		2-16MMØ		2-16MMØ		12@0.05, REST@0.10
ROOF	RB-1	0.20	20 0.30	WEB BAR					10MMØ	
				BOTTOM BAR		2-16MMØ		2-16MMØ		
	B-1		0.25 0.35	TOP BAR		3-16MMØ		2-16MMØ	10MMØ	14@0.05, REST@0.125
		0.25		WEB BAR				ST-EN		
BOARDWALK				BOTTOM BAR		2-16MMØ		2-16MMØ		
LEVEL		0.20 0.3	0.20 0.30	TOP BAR		2-16MMØ		2-16MMØ		12@0.05, REST@0.10
	B-2			WEB BAR					10MMØ	
			A DESTRU	BOTTOM BAR		2-16MMØ		2-16MMØ		

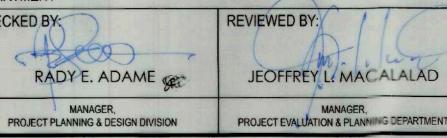






PROJECT TITLE/ LOCATION	P
CONSTRUCTION OF THE MANGROVE FOREST TRAIL AND CONSERVATION CENTER	P
	D
ARTECHE FASTERN SAMAR	U





RECOMMENDING APPROVAL:
00404
CREGORY A. OLLER
ASSISTANT CHIEF OPERATING OFFICER, ARCHITECTURAL & ENGINEERING SERVICES SECTION

	2 Aringhetes.	
	MARKT. LAPID	100
}	CHIEF OPERATING OFFICER	H

APPROVED BY:

SHEET CONTENTS: NTS R.A. 9266

TYPICAL BEAM DETAILS,
TYPICAL BEAM
REINFORCEMENTS DETAILS,
TYPICAL COLUMN DETAILS,
TYPICAL COLUMN TIE
ARRANGEMENT DETAILS,
TYPICAL FOOTING PLAN,
TYPICAL FOOTING DETAILS,
REVISIONS:

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